

## GENERAL

. THE TYPICAL NOTES SHALL APPLY FOR ALL CASES UNLESS OTHERWISE SPECIICCALYY DETAILED WITHIN THE DRAWINGS. SOME NOTES MAY NOT BE APPLCABLE IN PART OR IN WHOLE FOR EVERY PROJECT.
any ttems referenced as being on "Hold" are to be included in the work as shown However, construction or fabrication is not to begin until the "Hold" reference is EEMOVED.
dimensions contained within must be field verified and customer approved prior to FAbrication of materials.
THE MODIFCCATIONS DEPICTED in THESE DRAWINGS ARE INTENDED TO PROVIDE STRUCTURAL suport for the addition of the antenna screening systems outuned within. The exiting STRUCTURE OR BUILDING SHALL BE ANALYZED AND RETROFITTED AS REQUIRED, BY OTHERS, TO
ANTAN THN LOADS IMPOSED BY THE NEW STEALTH® ENCLLOSURE SHOWN ON THE DRAWINGSS.
AROCTS SHALL BE INSTALLED BY A CONTRACTOR EXPERIENCED IN 5. ANTENNA CONCEALMENT PRODUCTS SHALL BE INSTALLED BY A CONTRACTOR EXPERIENCED IN
SIMLLAR WORK. CARE SHALL BE TAKEN IN THE INSTALLATION OF ANY AND ALL MEMBERS IN ACCORDANC IILAR WORK. CARE SHALL BE TAKEN IN THE INSTALATION OF ANY AND ALL MEM BERS IN ACCORDANC ARE TO BE FOLOWED. STEALTH® IS NOT PROVIIING FIELD INSTALLATION SUPERVIIIION.
THESE DRAWINGS INDICATE THE MAOR OPERATIONS TO BE PERFORMED, BUT DO NOT SHOW IVERY FIELD CONDITION THAT MAY BE ENCOUNTERED. THEREFORE PRTOR TO BEGINING OF WORK THE Contractor should survey the job site thoroughly to minimize field problems.
PROTECTION OF EXISTING STRUCTURES DURING THE COURSE OF THE CONSTRUCTION SHALL BE THE RESPONSIBILTTY OF THE GENERAL CONTRACTOR.
8. THE STRUCTURAL INTEGRITY OF THIS STRUCTURE IS DESIGNED TO BE ATTAINED IN ITS COMPLLTED STATE. WHILE UNDER CONSTRUCTION ANY TEMPORARY BRACING OR SHORING WHICH MAY BE EQuired to maintain stabilut prior to completion shall be the responsiblity of the eneral contractor.
and details within do not include details or design for drainage from oin deting of Exterior or interior surfaces of the existing builoing or structure.

## aterial notes:

1. 18-IIDED MONOPOLE SHAFT STEEL SHALL CONFORM w/ ASTM A572 GR. 65 , U.N.O.
2. BASE PLATE STEEL SHALL CONFORM W/ ASTM A572, GR. 50, U.N.O
3. REINFORCED ACCESS PORT STEEL SHALL CONFORM W/ ASTM A572 GR. 65 , U.N.O.
4. ALL STEEL TUBES (SQUARE \& RECTANGULAR) SHALL CONFORM w/ ASTM A500 GR. C ( 50 ksi) , U.N.O 5. ALL OTHER STRUCTURAL STEEL SHAPES \& PLLTES SHALL CONFORM TO ASTM AB6, U.N.O.
. ALL BLLTS FOR THE STEEL-TO-STEEL CONNECTIONS SHALL CONFORM W/ ASTM F3125 GR. A325, U.N.O. ALL WELDING SAALL BE PEEORMED BY CERIIIED WELDERS IN ACCORDANCE IN A SHOP APPROVED BY THE BUILDING OFFICIAL. STEEL WELDS SHALL BE BY ETOXX LOW HYDROGEN ELLECTRODES.
aL Steel surfaces shall be thoroughly coated with a zinc-rich primer or equivalent 9. ALL BOLTED CONNECTIONS SHALL BE TIGHTENED PER THE "TURN-OF-NUT" METHOD AS DEFINED BY AISC

## ISCLAIMERS:

1. ALL STRUCTURAL COMPONENTS TO BE CONNECTED TOGETHER SHALL BE COMPLLTELY Hit UP ON THE GROUND OR OTHERWISE VERIIED FOR COMPATIBIUTY PRIOR TO LITING ANY


## Stealthskin panels

FASTENER HOLES IN STEALTHKKIN FOAM COMPOSITE PANELS ARE NOT FACTORY DRILLED and must be drilled in the fielo.
PANEL FASTENERS TO BE SPACED 12" O.C. MAX. AND LOCATED $6^{\prime \prime}$ MAX. HORIZONTALLY FROM ach edgeat top and bottom of panel. Maintain $1 / 2^{2}$ Min. EDGe distance rrom all edges. 4' 'IID PANELS REQUIRE (4) FASTENERS TOP AND BOTTOM. 5 ' WIDE PANELS REQUIRE (5) FASTENERS TOP AND Bottom
WHEN FASTENER BOLT HEAD OR NUT BEARS DIRECTLY ON SURFACE OF STEALTHSKIN PANEL, TIGHTEN PANEL BOLTS ONLY Y// TURN PAST SNUG. APPLY THREAD LOCK COMPOUND TO THE STEALTH® STAIILLESS STEEL PANEL BOLTS USE WASHER OR FLANGED HEAD BOLT OR EASTENER with Large bearing surface.
4. PANELS WILL EXPAND AND CONTRACT DUE TO TEMPERATURE. WHEN INSTALLNG PANELSIN COLD TEMPERATURES, EVENLY SPACE PANELS ALONG LENGTH OF SCREEN WALL WITH EQUAL GAPS between panels to alow for expansion during warm temperatures.
5. adjacent flat panels are joined by a vertical foam spline that is inserted into Grooves cut into the side of each panel. do not LIT PANELS By grooves. panels Must be LITTED WITH FORCE DIRECTED ONTO PANEL SURFACE:
A. ADJACENT RADIUS PANELS ARE JoINED BY A VERTICAL H-CHANNEL. INSERT PANELS INTO H SIDE OF H-CHANNEL
RADIUS PANELS MUST BE EVENLY SPACED ALONG RADIUS SUPPORT. CONTRACTOR TO measure length of radius support and divide by the number of radius panels to
DETTRRIINE PROPER SPACING. H-CHANNEL CONNECTORS ARE USED TO COVER THE GAP BETWEEN panels and to allow for panel expansion and contraction.
8. SURFACES OF PANELS SHALL BE COATED WITH SUITABLE PAINT FOR UV PROTECTION. TO EDGE OF PANEL MUST BE COVERED TO PREVENT WATER TRAVEL BETWEEN PANELS. USE SHERWII WILLAMS "COROTHANE II" OR PRE APPROVED EQUIVALENT.
9. EXPOSED TOP AND SIDE FOAM EDGES OF PANELS MUST BE COVERED OR COATED FOR UV FOR MOST APPLCATOONS PANEL EDGE CAPS TO BE SECURED WITH TEK SCREW INSTALI ED @ 12 " maximum Spacing on the inside face of the panel.

## design notes:

STRUCTURAL DESIGN IS BASED ON THE 2012 NORTH CAROLINA BUILDING CODE (2009 IBC) \& THE ASCE 7 -05 STANDARD.

## DESIGN LOADS:

WIND:
( WIND SPEED: 90 MPH ( 3 -SEC GUST) PER TIA-222-G STANDARD
IMPORTANCE FACTOR: 1.00
RUCTURE CLASS / OCCUPANCY CATEGORY: I
EXPOSURE: C
OPOGRAPHIC CATEGORY: 1
ICE: 0.75" RADIAL ICE THICKNESS @ 30 MPH ( 3 -SEC GUST)
ESTTMATED WEIGHT (INCLUDING ANTENNAS AND EQUPMMENT): 47.8 k ( 1.0 DEAD)

REACTIONS:
SHEAR, $\mathrm{V}=31.7 \mathrm{k}$ ( 1.6 WIND)
MOMENT, $\mathrm{M}=3.721 \mathrm{k}$-f ( 1.6 WIND)
The reactions v \& M LSTED ABOVE SHALL Be CONSIDERED TO ACT in
ANY HORIZONTAL DIRECTION.

## DESIGN:

1. Engineering and design calculations for stealte® pole and tower product are . ARE DESIGNED
CLENT INPUT

## STRUCTURAL STEEL

. STEEL FABRICATTON AND INSTALLATION SHALL BE DONE IN ACCORDANCE WITH THE MERICAN INSTTTUTE OF STEEL CONSTRUCTION MANUAL AND SPECIIICATIONS,
STEEL I-SHAPE, ANGLE, CHANNEL, AND MISCELLANEOUS MEMBERS IONS, u.v.O
. SECIICATIONS U.N.O.
STEEL PIPE AND ROUND TUBE MEMBERS SHALL CONFORM TO ASTM A500 GRADE B (42 KS.
inN. Yield strengTh) steel specifications, un.o.
5. Steel rectangular and square tube memberr shall conform to astm amoo grade b 46 KSI MIN. YieLD STRENGTH) STEEL SPECIFICATIONS, U.N.O
STRENGTH) STEEL SPECCIICATIONS U.N.O. -
. BOLTS SHALL BE DOMESTIC, NEW HIGH STRENGTH GALVANIZED BOLTS, BEARING TYP
"X" (THREADS EXCLUDED), U.N.O., AND SHALL CONFORM TO ASTM A325 SPECCIFCATIONS, U.N.O
STRUCTURAL BOLTS SHALL BE TIGHTENED USING TURN-OF-THE-NUT METHOD
BOLT HOLE EDGE DISTANCES SHALL BE A MINIMUM $1^{\prime \prime}$, U.N.
10. ALL WELLING SHALL BE PERFORMED IN ACCORDANCE WTTH THE SPECIHCATIONS AND OR STEEL AND AWS DDI.2 FOR ALUMINUM. STEEL WELDS SHALL BE BY ETOXX, LOW HYRROGEN ELECTRODE.

1. STEEL SHALL BE HOT DIP galvanized PER ASTM A123 Specifications After fabrication OR PAINTED WITH RUST INHibitive PRIMER.
2. STEEL HARDWARE SHaLL BE HOT DIP GaLVANIZED PER ASTM A153, U.N.o
3. AFTER ANY FIELD HOLE PUNCHING / DRILLING OR CUTTING HAS BEEN COMPLETED, OR FOR信 THE MEMBERS.
4. aLL weLded steel assembles and individual steel parts should have the part NuMBER WELDED ONTO THE PART OR ASSEMBLY. THE PART NUMBERS SHOULD BE LOCATED Consistently and away from any connection point to avoid any interrerence issues with the weld.
STEALTH


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## SPECIAL INSPECTIONS \& STRUCTURAL OBSERVATION

1. STEEL FABRICATION SHALL BE DONE ON THE PREMISES OF A FABRICATOR REGITERED AND APRROVED AS REQUIRED BY THE BUILDING CODE TO PERFORM SUCH WORK WITHOUT SPECIAL inspection.
No Field welding shall be pernitted.
Chapter 17 OF THE BuILIING CODE

- PERIODIC SPECIAL INSPECTIION OF HIGH-STRENGTH BOLTING

Continuous special inspection of driling operations for pier foundations CONTINUOUS SPECIAL INSPECTION TO VERIF LOCATION, PLUMBNESS, DIAMETER, AND
PERIODIC SPECIAL INSPECTION TO VERIFY ADEQUATE SoIl BeLow GRadE

- CONTINUOUS SPECIAL INSPECTION OF ANCHOR BOLTS PRIOR TO AND DURING CONCRETE PLACEMENT

4. SPECIAL INSPECTTON IS NOT REQURED FOR WORK OF A MINOR NATURE OR AS WARRANTED SY CONDITTONS IN THE JURISDICTTON AS APPROVED BY THE BULLDING OFFICIAL. THUS, SPECIAL 5. no structural observation is required.


04/06/17


|  |  |  | REVISION TABLE |
| :---: | :---: | :---: | :---: |
| Revision | DESIGNER | DATE | SCOPE OF REVISION |
| 0 | TAJ-VSE | 4-6-17 | FINAL ENGINEERING |
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2. ALL CONCRETE SHALL USE TYPE II PORTLAND CEMENT AND HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4000 PSI AT 28 DAYS. CONCRETE SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.50. CONCRETE SHALL HAVE A SLUMP OF 5 " ( $\pm$ ") OR AS SPECIFIED BY THE GEOTECHNICAL ENGINEER. ALL CONCRETE WORK SHALL BE CCORDANCE WITH "THE BUILDING CODE REQUIREMENTS FOR REINFORCED ACI 336 , "STANDARD SPECIICATIONS FOR THE CONSTRUCTION OF DRILED PIERS," LATEST EDITION.
3. Reinforcing steel shall conform with the requirements of astm a-615, GRADE 60. ALL REINFORCING DETAILLS SHALL CONFORM TO "MANUAL O F STANDAR RACTICE FOR DETALLING REINFORCED CONCRETE STRUCTURES,' ACI 315, LATEST

INSTALLATION OF DRILED PIERS MUST BE OBSERVED BY A REPRESENTATVE O the geotechnical engineer frm. geotechnical engineer to provide a NOTICE OF INSPECTION FOR THE BUILDING INSPECTOR FOR REVIEW AND RECORD URPOSES.
5. ALL ANCHOR BOLTS SHALL CONFORM w/ ASTM A615 GR. 75, GALVANIZED, U.N.O.




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